



Title	On the Flexural Deflection of a Moderately Thick Plate : Part II. Solution of Equation for Deflection of the Plate
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Citation	Memoirs of the Faculty of Engineering, Hokkaido University, 15(3), 367-378
Issue Date	1981-01
Doc URL	<a href="http://hdl.handle.net/2115/37990">http://hdl.handle.net/2115/37990</a>
Type	bulletin (article)
File Information	15(3)_367-378.pdf



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# On the Flexural Deflection of a Moderately Thick Plate

## Part II. Solution of Equation for Deflection of the Plate

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(Received June 28, 1980)

### Résumé

The equation of deflection for a moderately thick plate presented in our previous paper, is solved for two cases, namely, a) a simply supported rectangular plate under distributed pressure, and b) a simply supported circular plate under uniformly distributed pressure.

Results obtained here are used to calculate the maximum deflection of the plate, and are compared with the results hitherto obtained.

### § 1. Notations and Equation of Flexural Deflection of a Moderately Thick Plate

#### Notations

$x_i$ : rectangular coordinates, ( $i=1, 2, 3$ )

$\xi_i$ : components of displacement, ( $i=1, 2, 3$ )

$\varepsilon_{ij} = \frac{1}{2} \left( \frac{\partial \xi_j}{\partial x_i} + \frac{\partial \xi_i}{\partial x_j} \right)$ : components of strain, ( $i, j=1, 2, 3$ )

$\varepsilon_{kk} = \varepsilon_{11} + \varepsilon_{22} + \varepsilon_{33}$ ,

$A_{ij} = \lambda \varepsilon_{kk} \delta_{ij} + 2\mu \varepsilon_{ij}$ : components of stress, ( $i, j=1, 2, 3$ )

with Lamé's constants  $\lambda$  and  $\mu$ ,

$(l, m) = l\lambda + m\mu$ ,  $(l, m, n) = l\lambda^2 + m\lambda\mu + n\mu^2$ , etc.

$h$ : thickness of plate,

$D = \frac{h^3(0, 1)(1, 1)}{3(1, 2)}$ : flexural rigidity of plate, and

$w_0$ : deflection of plate, *i. e.* vertical displacement of the middle plane of plate.

We shall take  $x_1$ - and  $x_2$ -axes on the middle plane of the plate,  $x_3$ -axis being directed downwards.

#### Equation of Flexural Deflection of a Moderately Thick Plate

In the previous paper<sup>1)</sup>, the fundamental equations for deflection of a moderately thick plate were presented, and it was shown that one can obtain approximate

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equations with any desired accuracy after considering that the thickness of plate is small. We shall summarize the results here.

Expanding components of displacement  $\xi_i$  ( $i=1, 2, 3$ ) of the plate into power series in  $x_3$ :

$$\left. \begin{aligned} \xi_1 &= \sum_{k=0}^{\infty} u_{2k+1} x_3^{2k+1}, \\ \xi_2 &= \sum_{k=0}^{\infty} v_{2k+1} x_3^{2k+1}, \\ \xi_3 &= \sum_{k=0}^{\infty} w_{2k} x_3^{2k}, \end{aligned} \right\} \quad (1-1)$$

and introducing (1-1) into equations of equilibrium of an elastic body:

$$0 = \frac{\partial A_{ij}}{\partial x_j} = (1, 1) \frac{\partial}{\partial x_i} \varepsilon_{kk} + (0, 1) \Delta_3 \xi_i, \quad (i = 1, 2, 3) \quad (1-2)$$

with

$$\Delta_3 = \frac{\partial^2}{\partial x_k^2} = \Delta + \frac{\partial^2}{\partial x_3^2},$$

we obtain the following relations\* among the coefficients of power series in eqs. (1-1):

$$\left. \begin{aligned} u_{2k+1} &= \frac{(-1)^k}{(2k+1)!} \left[ \Delta^k u_1 + \frac{k(1, 1)}{(1, 2)} \Delta^{k-1} \frac{\partial}{\partial x_1} \{ \Xi_1 - \Delta w_0 \} \right], \quad (k=0, 1, 2, \dots) \\ v_{2k+1} &= \frac{(-1)^k}{(2k+1)!} \left[ \Delta^k v_1 + \frac{k(1, 1)}{(1, 2)} \Delta^{k-1} \frac{\partial}{\partial x_2} \{ \Xi_1 - \Delta w_0 \} \right], \quad (k=0, 1, 2, \dots) \end{aligned} \right\} \quad (1-3)$$

and

$$w_{2k} = \frac{(-1)^k}{(2k)!} \left[ \Delta^k w_0 + \frac{k(1, 1)}{(1, 2)} \Delta^{k-1} \{ \Xi_1 - \Delta w_0 \} \right], \quad (k=0, 1, 2, \dots)$$

with

$$\Xi_1 = \frac{\partial u_1}{\partial x_1} + \frac{\partial v_1}{\partial x_2}. \quad (1-4)$$

Boundary conditions at the surfaces of the plate read:

$$\left. \begin{aligned} \pm \frac{p}{2} &= A_{33} = (1, 0) \varepsilon_{kk} + 2(0, 1) \varepsilon_{33}, \quad \text{at } x_3 = \pm \frac{h}{2} \\ 0 &= A_{31} = 2(0, 1) \varepsilon_{31}, \quad \text{at } x_3 = \pm \frac{h}{2} \\ 0 &= A_{32} = 2(0, 1) \varepsilon_{32}, \quad \text{at } x_3 = \pm \frac{h}{2} \end{aligned} \right\} \quad (1-5)$$

where  $p=p(x_1, x_2)$  is the distributed external pressure over the upper surface of the plate. Introducing (1-1) with (1-3) into (1-5), we have the following equations:

\* We understand that the zero-th power of the Laplacian operator is equal to unity, *i. e.*  $\Delta^0=1$ .

$$\frac{4}{3}(1, 1) p = D \sum_{k=0}^{\infty} \frac{(-1)^k (2k+2)}{(2k+3)!} \left(\frac{h}{2}\right)^{2k} \left[ (2k+1, 2k) \Delta^{k+2} \tau w_0 - (2k+3, 2k+4) \Delta^{k+1} \Xi_1 \right], \quad (1-6)$$

$$\sum_{k=0}^{\infty} \frac{(-1)^k}{(2k)!} \left(\frac{h}{2}\right)^{2k} \left[ (2k+1, 2k+2) \Delta^k \Xi_1 - (2k-1, 2k-2) \Delta^{k+1} \tau w_0 \right] = 0, \quad (1-7)$$

and

$$\sum_{k=0}^{\infty} \frac{(-1)^k}{(2k)!} \left(\frac{h}{2}\right)^{2k} \Delta^k \Omega_1 = 0, \quad (1-8)$$

with flexural rigidity  $D$  of the plate of thickness  $h$ :

$$D = \frac{h^3(0, 1)(1, 1)}{3(1, 2)},$$

and

$$\Omega_1 = \frac{\partial u_1}{\partial x_2} - \frac{\partial v_1}{\partial x_1}. \quad (1-9)$$

When the thickness of plate is small compared with its lateral dimensions, one can find:

$$\Omega_1 = \frac{\partial u_1}{\partial x_2} - \frac{\partial v_1}{\partial x_1} = 0. \quad (1-10)$$

Eq. (1-10) shows that there is a function  $\phi$  such that:

$$u_1 = \frac{\partial \phi}{\partial x_1}, \quad \text{and} \quad v_1 = \frac{\partial \phi}{\partial x_2}, \quad (1-11)$$

with

$$\Xi_1 = \Delta \phi. \quad (1-12)$$

From (1-7)~(1-12),  $\phi$  is expressed in the form:

$$\begin{aligned} \phi = & -\tau w_0 - \frac{2(1, 1)}{(1, 2)} \left(\frac{h}{2}\right)^2 \Delta \tau w_0 - \frac{2(1, 1)(4, 5)}{3(1, 2)^2} \left(\frac{h}{2}\right)^4 \Delta \Delta \tau w_0 - \\ & - \frac{2(1, 1)(27, 68, 43)}{15(1, 2)^3} \left(\frac{h}{2}\right)^6 \Delta \Delta \Delta \tau w_0 - \dots \end{aligned} \quad (1-13)$$

Eliminating  $\Xi_1$  from (1-6) by means of (1-12) and (1-13), we obtain:

$$\begin{aligned} p = D \Delta \Delta \left\{ \tau w_0 + \frac{(13, 16)}{10(1, 2)} \left(\frac{h}{2}\right)^2 \Delta \tau w_0 + \frac{(1479, 3704, 2332)}{840(1, 2)^2} \left(\frac{h}{2}\right)^4 \Delta \Delta \tau w_0 + \right. \\ \left. + \frac{(35969, 135768, 171420, 72400)}{15120(1, 2)^3} \left(\frac{h}{2}\right)^6 \Delta \Delta \Delta \tau w_0 + \dots \right\}. \end{aligned} \quad (1-14)$$

Retaining terms of  $O(h^{2n})$  in the right-hand side of eqs. (1-13) and (1-14), we obtain the equation for deflection of a moderately thick plate in the  $n$ -th order approximation in our theory. We shall cite here merely the equations in the zero-th,

the first, the second, and the third order approximations. For the sake of simplicity, we shall omit the subscript of  $w_0$  hereafter, writing  $w$  instead of  $w_0$ .

A) *Zero-th order approximation*

$$\phi = -w, \quad (1-15)$$

and

$$p = D\Delta\Delta w. \quad (1-16)$$

B) *First order approximation*

$$\phi = -w - \frac{2(1, 1)}{(1, 2)} \left(\frac{h}{2}\right)^2 \Delta w, \quad (1-17)$$

and

$$p = D\Delta\Delta \left\{ w + \frac{(13, 16)}{10(1, 2)} \left(\frac{h}{2}\right)^2 \Delta w \right\}. \quad (1-18)$$

C) *Second order approximation*

$$\phi = -w - \frac{2(1, 1)}{(1, 2)} \left(\frac{h}{2}\right)^2 \Delta w - \frac{2(1, 1)(4, 5)}{3(1, 2)^2} \left(\frac{h}{2}\right)^4 \Delta\Delta w, \quad (1-19)$$

and

$$p = D\Delta\Delta \left\{ w + \frac{(13, 16)}{10(1, 2)} \left(\frac{h}{2}\right)^2 \Delta w + \frac{(1479, 3704, 2332)}{840(1, 2)^2} \left(\frac{h}{2}\right)^4 \Delta\Delta w \right\}. \quad (1-20)$$

D) *Third order approximation*

$$\begin{aligned} \phi = -w - \frac{2(1, 1)}{(1, 2)} \left(\frac{h}{2}\right)^2 \Delta w - \frac{2(1, 1)(4, 5)}{3(1, 2)^2} \left(\frac{h}{2}\right)^4 \Delta\Delta w - \\ - \frac{2(1, 1)(27, 68, 43)}{15(1, 2)^3} \left(\frac{h}{2}\right)^6 \Delta\Delta\Delta w, \end{aligned} \quad (1-21)$$

and

$$\begin{aligned} p = D\Delta\Delta \left\{ w + \frac{(13, 16)}{10(1, 2)} \left(\frac{h}{2}\right)^2 \Delta w + \frac{(1479, 3704, 2332)}{840(1, 2)^2} \left(\frac{h}{2}\right)^4 \Delta\Delta w + \right. \\ \left. + \frac{(35969, 135768, 171420, 72400)}{15120(1, 2)^3} \left(\frac{h}{2}\right)^6 \Delta\Delta\Delta w \right\}. \end{aligned} \quad (1-22)$$

## § 2. Solution of Equation of Deflection for a Simply Supported Rectangular Thick Plate

Eqs. (1-6)~(1-8) as well as equations in any order approximation can be solved for a simply supported rectangular plate under distributed pressure.

Let the rectangular plate occupy the region:  $-a/2 \leq x_1 \leq a/2$  and  $-b/2 \leq x_2 \leq b/2$ , and be simply supported at  $x_1 = \pm a/2$  and  $x_2 = \pm b/2$ . We shall take the boundary conditions in this case as follows:

$$\xi_2 = \xi_3 = 0, \quad \text{and} \quad A_{11} = 0, \quad \text{at} \quad x_1 = \pm a/2 \quad (2-1)$$

and

$$\xi_1 = \xi_3 = 0, \quad \text{and} \quad A_{22} = 0. \quad \text{at} \quad x_2 = \pm b/2 \quad (2-2)$$

Under the boundary conditions (2-1) and (2-2), solution of eqs. (1-6)~(1-8) can be expressed by :

$$w = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} W_{mn} \cos \frac{(2m-1)\pi}{a} x_1 \cdot \cos \frac{(2n-1)\pi}{b} x_2, \quad (2-3)$$

$$\Omega_1 = 0, \quad (2-4)$$

and

$$\phi = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} \Phi_{mn} \cos \frac{(2m-1)\pi}{a} x_1 \cdot \cos \frac{(2n-1)\pi}{b} x_2, \quad (2-5)$$

with (1-11) and (1-12).

The distributed pressure  $p = p(x_1, x_2)$  is expressed in a double Fourier series :

$$p = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} p_{mn} \cos \frac{(2m-1)\pi}{a} x_1 \cdot \cos \frac{(2n-1)\pi}{b} x_2, \quad (2-6)$$

with

$$p_{mn} = \frac{4}{ab} \int_{-\frac{b}{2}}^{\frac{b}{2}} \int_{-\frac{a}{2}}^{\frac{a}{2}} p(x_1, x_2) \cos \frac{(2m-1)\pi}{a} x_1 \cdot \cos \frac{(2n-1)\pi}{b} x_2 dx_1 dx_2. \quad (m, n=1, 2, 3, \dots) \quad (2-7)$$

When the pressure is uniformly distributed, *i. e.*  $p = \text{const} (=p_0)$ , expression (2-7) turns simply to be :

$$p_{mn} = \frac{2^4}{\pi^2} \frac{(-1)^{m+n}}{(2m-1)(2n-1)} p_0. \quad (m, n=1, 2, 3, \dots) \quad (2-8)$$

Introducing (2-3)~(2-6) with (1-12) into (1-6)~(1-8), we obtain :

$$W_{mn} = \frac{1}{6D} \frac{p_{mn}}{\gamma_{mn}^4} (\gamma_{mn} h)^3 \frac{\frac{(1,1)}{(1,2)} \left( \gamma_{mn} \frac{h}{2} \right) \sinh \left( \gamma_{mn} \frac{h}{2} \right) + \cosh \left( \gamma_{mn} \frac{h}{2} \right)}{\sinh(\gamma_{mn} h) - \gamma_{mn} h}, \quad (m, n=1, 2, 3, \dots) \quad (2-9)$$

$$\Phi_{mn} = \frac{1}{6D} \frac{p_{mn}}{\gamma_{mn}^4} (\gamma_{mn} h)^3 \frac{\frac{(1,1)}{(1,2)} \left( \gamma_{mn} \frac{h}{2} \right) \sinh \left( \gamma_{mn} \frac{h}{2} \right) - \cosh \left( \gamma_{mn} \frac{h}{2} \right)}{\sinh(\gamma_{mn} h) - \gamma_{mn} h}, \quad (m, n=1, 2, 3, \dots) \quad (2-10)$$

with

$$\gamma_{mn}^2 = \left[ \frac{(2m-1)\pi}{a} \right]^2 + \left[ \frac{(2n-1)\pi}{b} \right]^2. \quad (m, n=1, 2, 3, \dots) \quad (2-11)$$

By means of (2-3) and (2-9), we have the expression for  $w$ , which is found

to be identical with the result obtained by Iyenger et al.<sup>2)</sup> They obtained the solution of the equation derived from Vlasov's method<sup>3)</sup>.

If we express the right-hand side of eq. (2-9) into power series in  $h$  :

$$W_{mn} = \frac{1}{D} \frac{p_{mn}}{r_{mn}^4} \left[ 1 + \frac{(13, 16)}{10(1, 2)} \left( \gamma_{mn} \frac{h}{2} \right)^2 - \frac{(297, 454)}{4200(1, 2)} \left( \gamma_{mn} \frac{h}{2} \right)^4 + \dots \right],$$

( $m, n=1, 2, 3, \dots$ ) (2-12)

and truncate the series at the terms of  $O(h^{2n})$ , we can obtain the solution of equation in the  $n$ -th order approximation.

### § 3. Solution of Equation of Deflection for a Simply Supported Circular Thick Plate under Uniform Pressure

Let us take cylindrical coordinates  $r = \sqrt{x_1^2 + x_2^2}$ ,  $\theta = \arctg(x_2/x_1)$ , and  $z = x_3$ . Then, Laplacian operator  $\Delta$  reads :

$$\Delta = \frac{\partial^2}{\partial r^2} + \frac{1}{r} \frac{\partial}{\partial r} + \frac{1}{r^2} \frac{\partial^2}{\partial \theta^2},$$

which appears in eqs. (1-6)~(1-8), (1-16), (1-18), (1-20), etc.

Components of displacement and components of stress are to be written also in cylindrical coordinates :

$$\begin{aligned} \xi_r &= \xi_1 \cos \theta + \xi_2 \sin \theta, & \xi_\theta &= -\xi_1 \sin \theta + \xi_2 \cos \theta, & \xi_z &= \xi_3, \\ A_{rr} &= \lambda \varepsilon_{kk} + 2\mu(\partial \xi_r / \partial r), & \text{etc.} & & & \end{aligned}$$

with

$$\varepsilon_{kk} = \frac{1}{r} \frac{\partial(r\xi_r)}{\partial r} + \frac{1}{r} \frac{\partial \xi_\theta}{\partial \theta} + \frac{\partial \xi_z}{\partial z}.$$

We shall solve eqs. (1-16), (1-18), and (1-20) for a simply supported circular plate under *uniform* pressure  $p_0$  over the surface of the plate. Accordingly we have :

$$\xi_\theta = 0, \quad \frac{\partial}{\partial \theta} = 0, \quad \text{and} \quad \Delta = \frac{1}{r} \frac{d}{dr} \left( r \frac{d}{dr} \right).$$

Let the circular plate occupy the region:  $0 \leq r \leq a$ , and be simply supported at  $r = a$ . As for the boundary conditions, we shall take :

$$\xi_z = 0, \quad \text{and} \quad M_r \equiv \int_{-\frac{h}{2}}^{\frac{h}{2}} A_{rr} z dz = 0, \quad \text{at} \quad r = a \tag{3-1}$$

and let us start from the zero-th order approximation.

#### A) Zero-th Order Approximation

The solution  $w^{(0)}$  of equation for deflection in the zero-th order approximation (1-16), is expressed by a particular solution of (1-16), say  $w_1^{(0)}$ , plus a biharmonic function  $w_{11}^{(0)}$ , namely

$$w^{(0)} = w_1^{(0)} + w_{II}^{(0)}, \tag{3-2}$$

with

$$D\Delta\Delta w_1^{(0)} = p_0, \tag{3-3}$$

and

$$\Delta\Delta w_{II}^{(0)} = 0. \tag{3-4}$$

The solution of eq. (3-3) is given by :

$$w_1^{(0)} = \frac{1}{64} \frac{p_0}{D} r^4. \tag{3-5}$$

While, the solution of eq. (3-4), which is finite at  $r=0$ , reads

$$w_{II}^{(0)} = C_1 r^2 + C_0, \tag{3-6}$$

with constants  $C_1$  and  $C_0$ . Determining  $C_1$  and  $C_0$ , so as to satisfy the boundary conditions (3-1) for  $w^{(0)}$ , we obtain the solution of eq. (1-16) :

$$w^{(0)} = w_1^{(0)} + w_{II}^{(0)} = \frac{1}{64} \frac{p_0}{D} (a^2 - r^2) \left\{ \frac{(11, 10)}{(3, 2)} a^2 - r^2 \right\}. \tag{3-7}$$

Expression (3-7) is nothing but the usual solution for a circular thin plate.

*B) First Order Approximation*

The solution  $w^{(1)}$  of eq. (1-18) for deflection in the first order approximation is the sum of a particular solution of (1-18), say  $w_1^{(1)}$ , and the solution  $w_2^{(1)}$  of the homogeneous equation for eq. (1-18), namely

$$w^{(1)} = w_1^{(1)} + w_2^{(1)}, \tag{3-8}$$

with

$$D\Delta\Delta \left\{ w_1^{(1)} + \frac{(13, 16)}{10(1, 2)} \left( \frac{h}{2} \right)^2 \Delta w_1^{(1)} \right\} = p_0, \tag{3-9}$$

and

$$\Delta\Delta \left\{ 1 + \frac{(13, 16)}{10(1, 2)} \left( \frac{h}{2} \right)^2 \Delta \right\} w_2^{(1)} = \left\{ 1 + \frac{(13, 16)}{10(1, 2)} \left( \frac{h}{2} \right)^2 \Delta \right\} \Delta\Delta w_2^{(1)} = 0. \tag{3-10}$$

The solution  $w_2^{(1)}$  of eq. (3-10) is decomposed into two parts, namely

$$w_2^{(1)} = w_{II}^{(1)} + w_{III}^{(1)}, \tag{3-11}$$

where

$$\Delta\Delta w_{II}^{(1)} = 0, \tag{3-12}$$

and

$$\Delta w_{III}^{(1)} + a^2 w_{III}^{(1)} = 0, \tag{3-13}$$

with

$$\alpha^2 = \frac{10(1, 2)}{(13, 16)} \left( \frac{2}{h} \right)^2.$$

The solutions of eqs. (3-9) and (3-12) are :

$$w_I^{(1)} = \frac{1}{64} \frac{p_0}{D} r^4, \quad (3-14)$$

and

$$w_{II}^{(1)} = C_1 r^2 + C_0, \quad (3-15)$$

with constants  $C_1$  and  $C_0$ . While, the solution of eq. (3-13) is expressed by a Bessel function of order zero and is written as :

$$w_{III}^{(1)} = A J_0(\alpha r), \quad (3-16)$$

with a constant  $A$ .

Accordingly, the solution of eq. (1-18) under the boundary conditions (3-1) can be written by :

$$w^{(1)} = w_I^{(1)} + w_{II}^{(1)} + w_{III}^{(1)} = \frac{1}{64} \frac{p_0}{D} r^4 + C_1 r^2 + C_0 + A J_0(\alpha r), \quad (3-17)$$

with constants  $A$ ,  $C_1$ , and  $C_0$ , expressed as follows :

$$A = -\frac{(13, 16)}{80(3, 2)} a^4 \frac{p_0}{D} \left( \frac{h}{2a} \right)^2 \frac{1 + \frac{4(35, 66, 32)}{5(1, 2)^2} \left( \frac{h}{2a} \right)^2}{J_0(\alpha a) - \frac{4(1, 0)}{5(3, 2)} \alpha a \left( \frac{h}{2a} \right)^2 J_1(\alpha a)}, \quad (3-18)$$

$$C_1 = -\frac{1}{16} \frac{p_0}{D} a^2 + \frac{1}{4} a^2 A J_0(\alpha a), \quad (3-19)$$

and

$$C_0 = \frac{3}{64} \frac{p_0}{D} a^4 - \left( 1 + \frac{a^2 a^2}{4} \right) A J_0(\alpha a). \quad (3-20)$$

Expression (3-17) obtained here has a somewhat different feature from a solution given by Love<sup>4)</sup> :

$$w = \frac{1}{64} \frac{p_0}{D} (a^2 - r^2) \left\{ \frac{(11, 10)}{(3, 2)} a^2 - r^2 + \frac{8(35, 66, 32)}{5(1, 2)(3, 2)} \left( \frac{h}{2} \right)^2 \right\}, \quad (3-21)$$

where he took another boundary condition. Eq. (3-21), however, has a similar expression to (3-7) and also contains a term of  $O(h^2)$ .

### C) Second Order Approximation

In a similar manner, the solution  $w^{(2)}$  of eq. (1-20) in the second order approximation can be decomposed into three terms, namely

$$w^{(2)} = w_I^{(2)} + w_{II}^{(2)} + w_{III}^{(2)}, \quad (3-22)$$

with

$$D \Delta \Delta \left\{ w_I^{(2)} + \frac{2\alpha_2^2}{\beta_2^4} \Delta w_I^{(2)} + \frac{1}{\beta_2^4} \Delta \Delta w_I^{(2)} \right\} = p_0, \quad (3-23)$$

$$\Delta\Delta w_{II}^{(2)} = 0, \tag{3-24}$$

and

$$\Delta\Delta w_{III}^{(2)} + 2\alpha_2^2 \Delta w_{III}^{(2)} + \beta_2^4 w_{III}^{(2)} = 0, \tag{3-25}$$

with

$$\alpha_2^2 = \frac{42(1, 2)(13, 16)}{(1479, 3704, 2332)} \left(\frac{2}{h}\right)^2, \quad \text{and} \quad \beta_2^4 = \frac{840(1, 2)^2}{(1479, 3704, 2332)} \left(\frac{2}{h}\right)^4.$$

The solutions of eqs. (3-23) and (3-24) are given as :

$$w_I^{(2)} = \frac{1}{64} \frac{p_0}{D} r^4, \tag{3-26}$$

and

$$w_{II}^{(2)} = C_1 r^2 + C_0, \tag{3-27}$$

with constants  $C_1$  and  $C_0$ . While, the finite solution of (3-25) at  $r=0$  reads :

$$w_{III}^{(2)} = A_1 \Re I_0(\kappa r) + B_1 \Im I_0(\kappa r), \tag{3-28}$$

where  $I_0(\kappa r)$  is a modified Bessel function of order zero, with constants  $A_1$ , and  $B_1$ . We wrote one of the roots of the equation :

$$\kappa^4 + 2\alpha_2^2 \kappa^2 + \beta_2^4 = 0, \tag{3-29}$$

to be  $\kappa = \zeta + i\eta$  ( $\zeta \geq 0$  and  $\eta \geq 0$ ),

with

$$\zeta = \sqrt{\frac{\beta_2^2 - \alpha_2^2}{2}}, \quad \text{and} \quad \eta = \sqrt{\frac{\beta_2^2 + \alpha_2^2}{2}}. \tag{3-30}$$

The solution  $w^{(2)}$  of eq. (1-20) under the boundary conditions (3-1) can be written as :

$$\begin{aligned} w^{(2)} &= w_I^{(2)} + w_{II}^{(2)} + w_{III}^{(2)} = \\ &= \frac{1}{64} \frac{p_0}{D} r^4 + C_1 r^2 + C_0 + A_1 \Re I_0(\kappa r) + B_1 \Im I_0(\kappa r), \end{aligned} \tag{3-31}$$

with

$$A_1 = \frac{p_0}{D} a^4 \frac{\Im L(\kappa a) - \left\{ \frac{1}{8} + \frac{(17, 16)}{10(1, 2)} \left(\frac{h}{2a}\right)^2 \right\} \Im \{(\kappa a)^4 I_0(\kappa a)\}}{\Re L(\kappa a) \Im \{(\kappa a)^4 I_0(\kappa a)\} - \Im L(\kappa a) \Re \{(\kappa a)^4 I_0(\kappa a)\}}, \tag{3-32}$$

$$B_1 = -\frac{p_0}{D} a^4 \frac{\Re L(\kappa a) - \left\{ \frac{1}{8} + \frac{(17, 16)}{10(1, 2)} \left(\frac{h}{2a}\right)^2 \right\} \Re \{(\kappa a)^4 I_0(\kappa a)\}}{\Re L(\kappa a) \Im \{(\kappa a)^4 I_0(\kappa a)\} - \Im L(\kappa a) \Re \{(\kappa a)^4 I_0(\kappa a)\}}, \tag{3-33}$$

$$C_1 = -\frac{1}{16} \frac{p_0}{D} a^2 - \frac{1}{4a^2} [A_1 \Re \{(\kappa a)^2 I_0(\kappa a)\} + B_1 \Im \{(\kappa a)^2 I_0(\kappa a)\}], \tag{3-34}$$

and

$$C_0 = -\frac{1}{64} \frac{p_0}{D} a^4 - C_1 a^2 - A_1 \Re I_0(\kappa a) - B_1 \Im I_0(\kappa a), \tag{3-35}$$

where

$$\begin{aligned} L(\kappa a) = & (\kappa a)^2 I_0(\kappa a) - 2(\kappa a) I_1(\kappa a) + \frac{(17, 16)}{5(1, 2)} \left(\frac{h}{2a}\right)^2 \{(\kappa a)^4 I_0(\kappa a) - (\kappa a)^3 I_1(\kappa a)\} + \\ & + \frac{(1479, 3704, 2332)}{420(1, 2)^2} \left(\frac{h}{2a}\right)^4 \left\{ \frac{2(1, 1)}{(1, 2)} (\kappa a)^6 I_0(\kappa a) - \right. \\ & \left. - \frac{(1919, 4248, 2332)}{(1479, 3704, 2332)} (\kappa a)^5 I_1(\kappa a) \right\}. \end{aligned} \tag{3-36}$$

### § 4. Numerical Example

As for numerical examples, we shall consider the maximum deflection of a plate under uniform pressure calculated from solutions of several approximate equations obtained in this paper.

#### A) Rectangular Plate

Uniform pressure  $p=p_0$  (=const) is expressed in a double Fourier series in (2-7), giving

$$p_{mn} = \frac{2^4}{\pi^2} \frac{(-1)^{m+n}}{(2m-1)(2n-1)} p_0 \quad (m, n=1, 2, 3, \dots) \tag{4-1}$$

The maximum deflection  $w_{\max}$  of a rectangular plate is obtained from (2-3) with  $x_1=x_2=0$ , *i. e.*

$$w_{\max} = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} W_{mn}. \tag{4-2}$$

$W_{mn}$  are given in eq. (2-9) for the exact solution of the plate and in eq. (2-12) for approximate solutions.

For a square plate, *i. e.*  $a=b$ , we calculate  $w_{\max}$  for various values of  $h/a$ ,

TABLE 1. Comparison of the maximum deflections of a simply supported square plate ( $a=b$ ) under uniform pressure. ( $\lambda/\mu=3/2$ , and Poisson's ratio=0.3)

$h/a$	$w_{\max}/w_e$ ( $w_e$ : exact solution)		
	0-th order approx. (thin plate)	1st order approx.	Reissner <sup>5)</sup>
0.05	0.989	1.000	1.000
0.10	0.956	1.000	0.998
0.15	0.907	1.000	0.996
0.20	0.846	1.001	0.995
0.25	0.779	1.003	0.993
0.30	0.711	1.005	0.993

after truncating series (4-2) at appropriate terms ( $m, n \simeq 5 \sim 6$ ). In Table 1, the values  $w_{\max}/w_e$  are shown in the zero-th and the first order approximations in our theory and are compared with the values calculated from Reissner's theory<sup>6</sup>.  $w_e$  means the exact solution given by (4-2) with (2-9). The ratio  $\lambda/\mu$  is taken to be  $3/2$ , with Poisson's ratio  $(\lambda/2)/(\lambda+\mu)=0.3$ .

In the zero-th order approximation (*i. e.* thin plate), the relative error  $|1 - (w_{\max}/w_e)|$  is comparatively small (*e. g.* less than 5% for  $h/a=0.1$ ), while, for in creasing thickness, the error increases from *ca.* 9% (for  $h/a=0.15$ ) to *ca.* 30% (for  $h/a=0.3$ ). In the first order approximation in our theory, the values of  $w_{\max}$  agree very well with the exact solution, *e. g.* the error is 0.5% for  $h/a=0.3$ . In the second order approximation, the values of  $w_{\max}$  coincides very well with the exact values (*e. g.* the error is less than 0.1% for  $h/a \leq 0.4$ ).

### B) Circular Plate

The maximum deflection  $w_{\max}$  of a circular plate under uniform pressure is obtained from eqs. (3-7), (3-17), and (3-31), with  $r=0$ .

In the zero-th order approximation, we have :

$$w_{\max} = \frac{(11, 10)}{64(3, 2)} \frac{p_0}{D} a^4, \quad (4-3)$$

while the first order approximation gives :

$$w_{\max} = \frac{3}{64} \frac{p_0}{D} a^4 - \left(1 + \frac{\alpha^2 a^2}{4}\right) A J_0(\alpha a) + A, \quad (4-4)$$

with (3-18). The maximum deflection in the second order approximation reads :

$$w_{\max} = \frac{3}{64} \frac{p_0}{D} a^4 + A_1 + \left[ \frac{1}{4} \Re \{ (\kappa a)^2 I_0(\kappa a) \} - \Re I_0(\kappa a) \right] A_1 + \left[ \frac{1}{4} \Im \{ (\kappa a)^2 I_0(\kappa a) \} - \Im I_0(\kappa a) \right] B_1, \quad (4-5)$$

with (3-32) and (3-33).

Numerical results calculated from (4-3)~(4-5) and from Love's solution are compared in Table 2. In this case, the exact solution can not be obtained, and numerical values of  $w_{\max}$  are shown as a ratio of  $w_{\max}$  to  $w^{(0)}$ , for values of  $h/2a$ .  $w^{(0)}$  means the values in the zero-th order approximation (thin plate). The ratio  $\lambda/\mu$  is taken to be  $3/2$  and Poisson's ratio 0.3.

For the plate of small thickness, the values  $w_{\max}/w^{(0)}$  in the first and the second order approximations and also in the theory of Love, do not differ very much. For example, the values of the difference  $|1 - (w_{\max}/w^{(0)})|$  are less than 10% for  $h/2a \leq 0.1$ . The value increases with increasing thickness of the plate.

Although the features of (3-31) in our second order approximation and (3-21) given by Love are quite different from each other, the numerical values given in our second order approximation (4-5) show a very good agreement with those obtained by (3-21) for all the values of  $h/2a$ . The maximum difference between them is merely less than 7%.

TABLE 2. Comparison of the maximum deflections of a simply supported circular plate under uniform pressure.  
( $\lambda/\mu=3/2$ , and Poisson's ratio=0.3)

$h/2a$	$w_{\max}/w^{(0)}$ ( $w^{(0)}$ : 0-th order approximation)		
	1st order approx.	2nd order approx.	Love <sup>4)</sup>
0.05	0.997	1.012	1.009
0.10	1.090	1.047	1.036
0.15	1.016	1.111	1.081
0.20	1.441	1.221	1.145
0.25	1.462	1.314	1.226
0.30	1.683	1.337	1.326

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